

1 **Economic, energy and carbon footprint assessment of integrated forward**
2 **osmosis membrane bioreactor (FOMBR) process in urban wastewater**
3 **treatment**

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5 For submission to

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7 **Environmental Science: Water Research & Technology**

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10 **Supplementary information**
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26 Summary: There are seven pages in the supporting information section (4 tables and 1 graph)

27 **Appendix A: Mass balance, CAPEX and OPEX parameters and assumptions**

28 **Table A1.** Characteristics of the influent wastewater

Parameters of the wastewater influent	Symbol	Values	Unit
Influent flow rate	Q_{Inf}	15 000	$m^3 \text{ day}^{-1}$
Biosolids concentration in influent	$X_{S,Inf}$	100	mg-COD L^{-1}
COD concentration in influent	$S_{COD,Inf}$	400	mg-COD L^{-1}
NH_4^+ concentration in influent	$S_{\text{NH},Inf}$	50	mg-N L^{-1}
NO_3^- concentration in influent	$S_{\text{NO},Inf}$	0	mg-N L^{-1}

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30 Other assumptions implemented for the mass balance calculations:

31 i. All of the units were operated with the steady-flow, therefore, there was no
32 accumulation within the system.

33 ii. The density of the wastewater was assumed to be constant at 1000 kg m^{-3} (The
34 contaminants in the influent were too small to be compared with the volume of
35 wastewater to cause a significant change in the density).

36 iii. Constant influent characteristics, component conversion in the bioreactor, digester and
37 AMOX reactor. Constant solid destruction in digester over variation of sludge age,
38 stoichiometric ratio, kinetics and temperature of the process (Table A2, supplementary
39 information).

40 iv. The fouling formation, propensity and characteristics were assumed to be constant,
41 hence, the chemical cleaning frequency was expected to be consistent. However, it was
42 expected the fouling on FO membrane was relatively loose and low in propensity, and
43 therefore, FO required fewer cleaning regimes compared to MF and RO membranes.
44 Intense chemical cleaning, using 3500 ppm sodium hypochlorite (NaOCl) and 2500
45 ppm citric acid, was conducted 4 times per year for MF and RO, but only twice per year
46 for the FO.

47 v. The flow of the recycle stream from the sludge handling units to the bioreactors were
48 assumed to be insignificant to affect the characteristics of the influent wastewater.

50 **Table A2.** Parameters of the mass balance calculations

Scenario	Unit	Operation Conditions/ Assumptions
All scenarios	Primary Treatment	Primary Settler All COD solids is removed and transported to digestion (sludge treatment) at 1%
	Sludge treatment	Thickener Biosolids dewatered to a concentration of 5%
		Anaerobic digester Retention time in digester: 16 days Conversion to electricity: 40% Temperature of vessel: 40% Safety margin 1%
		Belt press Biosolids dewatered to a concentration of 15%
A and B	MFAeMBR and FOAeMBR	MBR COD conversion: 0.99 NH ₄ ⁺ conversion: 0.99 NO ₃ ⁻ conversion 0.7 - 0.99 Biomass retention time: 30 days Autotrophs decay rate: 0.1 per day Heterotrophs decay rate: 0.2 per day Autotrophs yield rate: 0.24 g cell COD formed/ g NH ₄ ⁺ -N oxidized Heterotrophs yield rate: 0.67 g cell COD formed/ g COD removed (oxidized+assimilated)
All scenarios with MF application	MFAeMBR and MFAnMBR	MF unit Water flux recovery: 95% Rejection performance: COD = 50% Intense chemical cleaning: 4 times per year Concentration of chemicals used for cleaning: 3500 ppm NaOCl, 2500 ppm citric acid
Scenario B: MFAeMBR-RO	RO	RO unit Water flux recovery: 75% Rejection performance: COD = 100%, NH ₄ = 98%; NO ₃ ⁻ = 95% Intense chemical cleaning: 4 times per year Concentration of chemicals used for cleaning: 3500 ppm NaOCl, 2500 ppm citric acid
All scenarios with FO application	FOAeMBR and FOAnMBR	FO unit Water flux recovery: 80% Rejection performance: COD = 99%, NH ₄ ⁺ = 93%; NO ₃ ⁻ = 85% (experimental results) Intense chemical cleaning: 2 times per year Concentration of chemicals used for cleaning: 3500 ppm NaOCl, 2500 ppm citric acid
Scenario B: FOAeMBR-RO	RO	RO unit Water flux recovery: 50% Rejection performance: COD = 100%, NH ₄ ⁺ = 98%; NO ₃ ⁻ = 95%

		Intense chemical cleaning: 4 times per year Concentration of chemicals used for cleaning: 3500 ppm NaOCl, 2500 ppm citric acid
		AnMBR <i>Remarks: Due to the complexity of the anaerobic reactions; it has been simplified to acidogenesis and methanogenesis. The COD in the wastewater is assumed to be fully degradable.</i>
Scenario C	MFAnMBR and FOAnMBR	Conversion by acidogenesis: 1 Conversion by methanogenesis: 0.99 Biomass retention time: 40 days Acidogenesis decay rate: 0.02 per day Methanogenesis decay rate: 0.036 per day Acidogenesis yield rate: 0.07 g COD formed/ g COD removed Methanogenesis 0.076 g COD formed/ g COD removed
		PN/Anammox Conversion by autotrophs: 0.57 Conversion by anammox (MF): 0.26 Conversion by anammox (FO): 0.40 Biomass retention time: Indefinite Autotrophs decay rate: 0.2 per day Anammox decay rate: 0.00384 per day Autotrophs yield rate: 0.24 g COD formed/ g COD removed Anammox 0.164 g COD formed/ g COD removed

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52 Other assumptions implemented for CAPEX and OPEX calculations:

53 i. Other values or costs provided in \$USD by the suppliers were converted by assuming
54 constant exchange rate of \$USD 1.00 = \$AUD 1.33.

55ii. The detailed calculation of the pump installation (except for the membrane units; MF, FO
56 and RO) in the wastewater treatment plants were not considered, therefore, a higher
57 contingency value (50%) was applied to cover for the pump installation and operation.

58ii. The cost of the gas purging in the anaerobic membrane bioreactor (AnMBR) was not
59 significant to be compared to the other OPEX (e.g. aeration and mixing) hence, purging
60 cost was not included in the calculations.

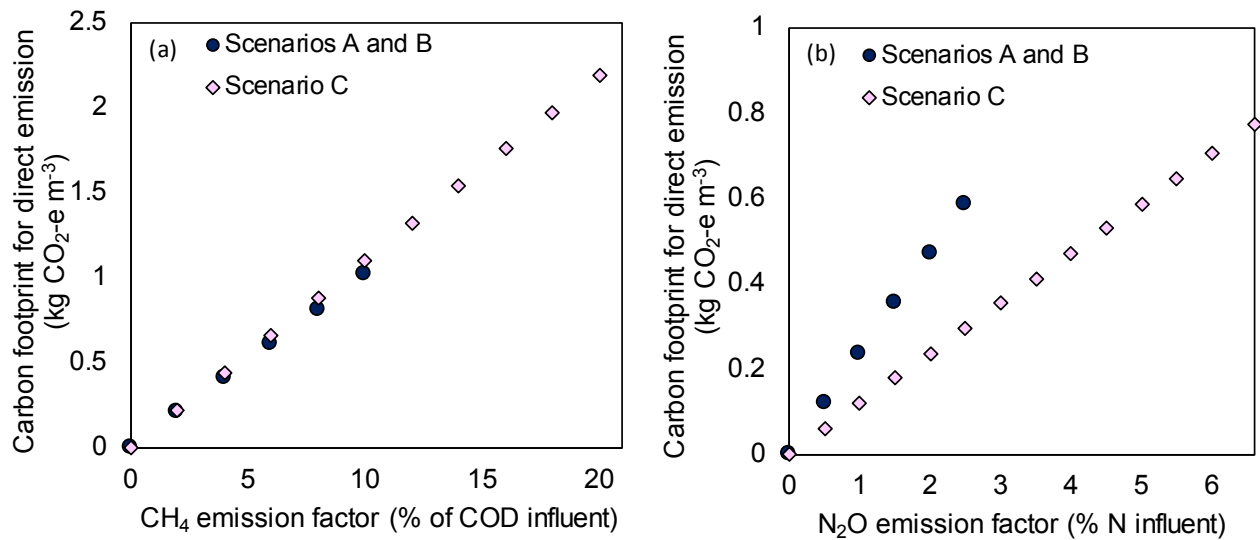
- 61iv. The energy consumed for the anaerobic digester unit included the energy required for
 62 stirring and pre-heating of the influent.
- 63v. Membrane cost was assumed to be constant throughout the operation. The installation and
 64 membrane replacement costs are as shown in Table 3 in the main manuscript.
- 65vi. The earnings of the wastewater treatment plant depending on the quality of the effluent or
 66 product water produced (Table A3, supplementary information).

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 68 **Table A3.** Specific earnings of wastewater treatment

Earnings		Values	Unit
Biological treatment	Carbon-based compounds removal	5	\$/ kg-chemical oxygen demand (COD)
	Nitrogen-based compounds removal	15	\$/kg-N
Electricity produced from methane		0.12	\$/kWh
Water sale		1.70	\$/m ³

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70 **Appendix B: Sensitivity analysis – carbon footprint**



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 72 **Fig. B1:** Sensitivity analysis of carbon footprint for direct emission due to the variation of
 73 GHG emit/mass influent ratio (a) CH₄ emission factor in scenarios A, B and C; (b) N₂O
 74 emission factor in scenarios A, B and C
 75

76 Based on 2006 IPCC Guidelines for National Greenhouse Gas Inventories¹, the CH₄ emission
 77 factor ($B_o \times MCF$) may vary from 5 – 10% and 5 – 20% of COD influent for aerobic-anoxic
 78 and anaerobic+PN/AMOX operations respectively under a sub-optimal operation. As for N₂O
 79 emissions, a review done by Law (2012) shows that the aerobic-anoxic may vary from from
 80 0.035 – 2.59 % of N-influent for aerobic-anoxic process and 0.4 – 6.6 % of N-influent for
 81 anaerobic+PN/AMOX process.²

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84 **Appendix C: Reverse salt diffusion baseline calculation**

85 **Table C1.** Methodologies of reverse salt diffusion in FOMBR operation

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Parameters	Equation/ Justification	Value	Unit
Volume of MBR (V_{MBR})	$\frac{\text{Mass biosolid in bioreactor}}{\text{Biosolid concentration}}$	896	m^3
Maximum salt concentration in MBR ($C_{\text{salt-max}}$)	Assumption	3	kg NaCl m^{-3}
Reverse salt flux (RSD_{flux})	Assumption	0.01	$\text{kg m}^{-2}\cdot\text{hour}^{-1}$
Area of MF required (A_{MF})	-	2776	m^2
Rate of salt diffuse (RSD_{rate})	$\text{RSD}_{\text{flux}} \times A_{\text{MF}}$	416	kg hour^{-1}
MF water flux (MF_{flux})	Assume the MF is operated at 2 bar	0.05	$\text{m}^3 \text{m}^{-2}\text{hour}^{-1}$

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88 The CAPEX and OPEX of MF in controlling salinity in FOMBR were referred to Choi's work
 89 (2015)³. The CAPEX of MF installation was \$119 944 meanwhile the CAPEX for the FO
 90 operation was \$31 504 year⁻¹. These values were equivalent to 5 % and 7 % of FO CAPEX
 91 cost and FO OPEX in scenario A respectively.

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94 **References:**

- 95 1 M. R. J. Doorn, S. Towprayoon, S. M. Manso Vieira, W. Irving, C. Palmer, R. Pipatti
96 and C. Wang, Wastewater Treatment and Discharge, *2006 IPCC Guidel. Natl. Greenh.*
97 *Gas Invent.*, 2006, 1–28.
- 98 2 Y. Law, L. Ye, Y. Pan and Z. Yuan, Nitrous oxide emissions from wastewater
99 treatment processes, *Philos. Trans. R. Soc. B Biol. Sci.*, 2012, **367**, 1265–1277.
- 100 3 Y. Choi, H. Cho, Y. Shin, Y. Jang and S. Lee, Economic evaluation of a hybrid
101 desalination system combining forward and reverse osmosis, *Membranes (Basel)*.,
102 2015, **6**, 1–19.
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